



August 1, 2025

Mr. Ryan Johnson
Town of Eagle
Public Works – Town Engineer
200 Broadway St
Eagle, CO 81631

RE: **Wildflower Cottages at Haymeadow**
Drainage Conformance Letter
HKS Project No. 2504026

Dear Mr. Johnson:

This letter summarizes how the proposed drainage improvements associated with the development of the Wildflower Cottages at Haymeadow ("Site"), conforms to the provisions allowed by the following studies:

- Haymeadow Filing 1 master drainage report, prepared September 2018 by Alpine Engineering, Inc. ("Report")
- Haymeadow Filing 2 – Phase 2 drainage conformance letter, prepared May 2025 by Harris Kocher Smith ("Letter")

The Site is more specifically Tracts RMF-4A and RMF-5, Haymeadow Filing Number 2, totaling 10.79 acres. It is bound by Mt Hope Circle to the south and east, Tract OS-B Haymeadow Filing Number 2 to the north, and Mt Hope Circle and Tract H Haymeadow Filing Number 1 to the west.

A copy of the vicinity map can be found in Appendix A.

The Site is proposed as a mix of single-family lots, duplexes and cottages, as well as amenities and open space areas. The Project also includes paved streets, surface parking, curb and gutter or gravel shoulders, hardscaping and landscaping.

The Site is currently undeveloped generally sloping from northeast to southwest. As shown in both the Report and Letter, the basins identified in Table 1 below are within the limits of disturbance of the Project and were designed for the ultimate build-out condition. Three storm mains, Storm Line B, Storm Line C and Storm Line D, were stubbed into the Site with the infrastructure that was constructed in Filing 1. Ultimately, Storm Lines B and C outfall into Pond 1B, and Storm Line D outfalls to Pond 1D, both ponds located to the south of the Site, south of Haymeadow Drive, previously Sylvan Lake Road.

Basin	Referenced Study	Area (acres)	C ₂₅	Q ₂₅	Q ₁₀₀	Outfall
B17P	Letter	0.61	0.40	0.78	1.43	Storm Line B
B18P	Letter	4.75	0.66	8.44	12.62	Storm Line B
D9P	Letter	5.72	0.68	8.43	12.37	Storm Line D
B11	Report	0.3	0.65	0.63	0.87	Storm Line B
B12	Report	0.08	0.3	0.08	0.13	Storm Line B
B14	Report	0.62	0.65	1.31	1.79	Storm Line B
B15	Report	0.27	0.65	0.57	0.78	Storm Line B
B16	Report	0.14	0.52	0.24	0.33	Storm Line B
C7	Report	0.57	0.65	1.2	1.64	Storm Line C
D8bP	Letter	1.11	0.66	2.35	3.51	Storm Line D

Table 1: Existing Basins within Project's Limits of Disturbance

On Site, runoff will be routed through the proposed curb and gutter, valley pans and on-site storm infrastructure to existing storm infrastructure within Whitney Peak Road and Mt Hope Circle to the south.

The Site is ultimately tributary to Brush Creek.

The Project modifies the existing referenced in Table 1 above into 19 sub-basins. The Rational Method was used to analyze the minor and major storm for the design of storm sewer. Runoff calculations for the developed conditions can be found in Appendix C. These modifications are summarized in Table 2.

Hydrologic calculations were done using the Rational Method per MHFD standards and can be found in Appendix C for reference. For this Drainage Compliance Letter, the 25-year storm event has been analyzed, as the Report uses the 25-year storm event as their controlling, design event.

REPORT/LETTER				PROPOSED			
BASIN	AREA	C ₂₅	Q ₂₅	BASIN	AREA	C ₂₅	Q ₂₅
MASTER REPORT STORM LINE B							
B14	0.62	0.65	1.31	B2	0.66	0.56	1.5
				B1	2.92	0.49	3.8
				B13P	0.71	0.46	1.2
B11	0.3	0.65	0.63	B16P	0.21	0.55	0.4
B12	0.08	0.3	0.08				
B16	0.14	0.52	0.24				
B15	0.27	0.65	0.57				
B17P	0.61	0.40	0.78	B17P2	0.58	0.40	0.8
B18P	4.75	0.66	8.44	B18P2	0.12	0.44	0.2
	6.77	0.63			5.20	0.48	
MASTER REPORT STORM LINE C							
C7	0.57	0.65	1.2	C1	1.46	0.52	2.4
				C2	1.04	0.64	2.3
				C6P	0.63	0.61	1.6
	0.57	0.65			3.13	0.58	
	7.34	0.63	13.25		8.33	0.52	14.2
MASTER REPORT STORM LINE D							
D9P	5.72	0.68	8.43	D9P2	0.77	0.48	1.1
				D1	0.53	0.65	1.4
				D2	0.82	0.59	1.6
				D3	0.32	0.53	0.7
				D4	0.46	0.70	1.5
				D5	0.27	0.48	0.5
				D6	0.37	0.77	1.3
				D7	0.46	0.62	1.2
				D8	0.58	0.53	1.0
D8bP	1.11	0.66	2.35	D8bP2	1.06	0.67	2.3
	6.83	0.66	10.78		5.63	0.61	12.6

HKS - Routed flows added to table for clarification of flows entering the existing storm sewer to better compare flows at design points between phases of the Site. Flows and C-Values are lower than designed in the Master Report and are in compliance.

Table 2: Comparison of Existing and Proposed

As shown in Table 2, a comparison of the C-values were utilized to show compliance with both the Report and the Letter. Weighted averages were used to compare the C-values of the subdivided basins against the basins from either the Report or Letter (in blue above).

Both the Report and the Letter show Basin B18 (Report) and Basin B18P (Letter) conveyed to existing Inlet B18 at the west side of the Site, when in actuality the majority of the flow from Basins B18 and B18P are split; a portion going to inlet B18 as designed, but a portion runs to Storm Line C.



given the increase in site flow and tributary area to the ponds the volume/flow of water when summed up appear be greater that the existing condition. Explain these effects on the downstream system

HKS - Site flow comparison updated to compare routed flows from subdivided conditions of existing basins from the Report and Letter. Since the conversion of the pond sizing between the Report and the Letter was influenced by peak flows and C-values, these parameters were used to justify compliance for the Site with this letter. Overall Routed flows and C-Values to Ponds 1B and 1D are now lower than existing. Explanation added to text regarding the changes in flows to the existing storm sewer from the Site.

pine and then outfall to Pond 1B, for the Lines B and C were combined and C-values calculated C values during the design storm the Site conforms to the Report and Letter.

ents to the existing basins throughout the can be found in Appendix D.

In an emergency event the Project is graded to release excess water to the south, towards Brush Creek, following previous studies.

The proposed development is not anticipated to have adverse impacts on upstream and downstream properties.

Please contact me if you have any questions or

On behalf of Harris Kocher Smith,

HKS - Narrative added describing new on-site storm sewer infrastructure along with software utilized to size pipes and inlets. Calculations added to Appendix C.

The report should also contain a narrative describing the new on-site storm-water conveyance infrastructure being added including discussion on inlet and storm pipe sizing. inlet capacity calculations need to be included in the report.

Rachel Patton, PE

References:

1. Drainage Report for Haymeadow Filing 1, Alpine Engineering, Inc., September 2018. "Report"
2. Haymeadow Filing 2 – Phase 2 Drainage Conformance Letter, Harris Kocher Smith, May 16, 2025. "Letter"
3. Urban Storm Drainage Criteria Manual, Vols. 1, 2 and 3, Mile High Flood District.

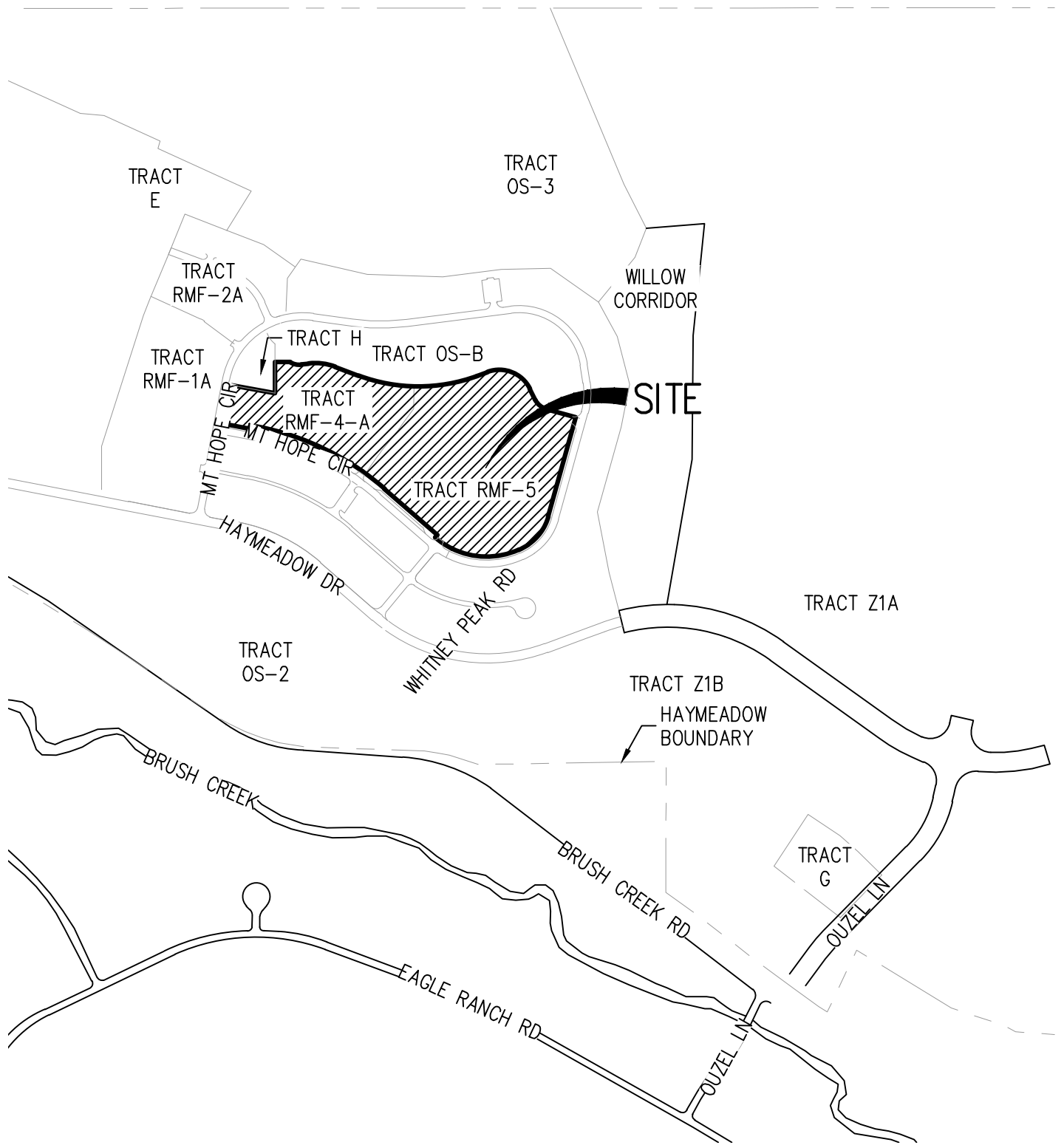
letter should be stamped

HKS - Stamp added to letter.



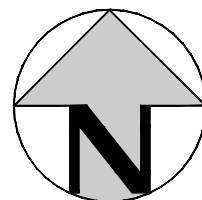
Appendix A – Vicinity Map

Plotted: MON 07/28/25 12:56:59P By: Olivia Riley Filepath: p:\2025\2504026\engineering\refvicmap-2504026.dwg Layout: layout1



VICINITY MAP

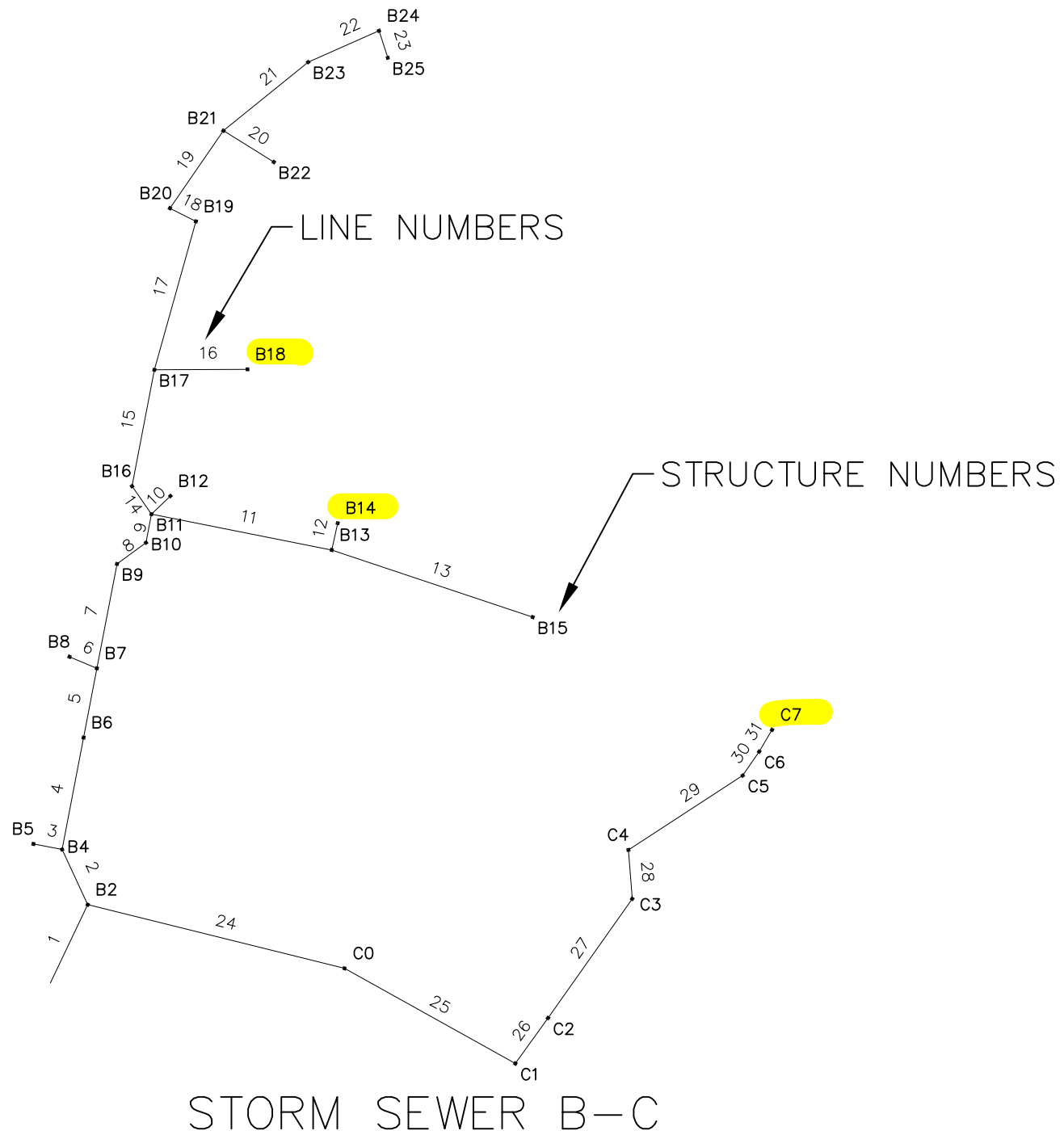
SCALE: 1"=1" = 500'



SCALE: 1" = 500'



Appendix B – Excerpts from Report & Letter



STORM RUN B
HAYMEADOW
RATIONAL METHOD SUMMARY

$$Q = C * I * A$$

BASIN	AREA (acres)	C ₂₅	I ₂₅	Q ₂₅ (cfs)	C ₁₀₀	I ₁₀₀	Q ₁₀₀ (cfs)
B2	0.64	0.60	3.25	1.25	0.65	4.12	1.71
B4	0.14	0.30	3.25	0.14	0.36	4.12	0.21
B5	0.19	0.54	3.25	0.33	0.58	4.12	0.46
B6	1.43	0.58	3.25	2.70	0.63	4.12	3.72
B7	0.12	0.55	3.25	0.21	0.60	4.12	0.30
B8	0.31	0.61	3.25	0.61	0.65	4.12	0.83
B9	0.05	0.75	3.25	0.12	0.78	4.12	0.16
B10	0.62	0.55	3.25	1.11	0.60	4.12	1.53
B11	0.30	0.65	3.25	0.63	0.70	4.12	0.87
B12	0.08	0.33	3.25	0.08	0.38	4.12	0.13
B13	0.13	0.67	3.25	0.28	0.71	4.12	0.38
B14	0.62	0.65	3.25	1.31	0.70	4.12	1.79
B15	0.27	0.65	3.25	0.57	0.70	4.12	0.78
B16	0.14	0.52	3.25	0.24	0.57	4.12	0.33
B17	0.72	0.26	3.25	0.61	0.32	4.12	0.95
B18	7.56	0.48	3.25	11.85	0.54	4.12	16.82
B19	0.14	0.63	3.25	0.29	0.67	4.12	0.39
B20	0.09	0.56	3.25	0.16	0.61	4.12	0.22
B21	0.07	0.46	3.25	0.11	0.51	4.12	0.15
B22	0.07	0.46	3.25	0.11	0.51	4.12	0.15
B23	0.99	0.36	3.25	1.14	0.41	4.12	1.69
B24	4.66	0.42	3.25	6.39	0.48	4.12	9.18
B25	0.06	0.65	3.25	0.13	0.69	4.12	0.17
TOTAL	19.40			30.38			42.89

STORM RUN C

HAYMEADOW

RATIONAL METHOD SUMMARY

$$Q = C * I * A$$

[illegible]

found in the Phase 1 conformance letter.

TABLE 1							
PER REPORT				PROPOSED			
BASIN	C ₂₅	Q ₂₅ (CFS)	Q ₁₀₀ (CFS)	BASIN	C ₂₅	Q ₂₅ (CFS)	Q ₁₀₀ (CFS)
B17	0.26	0.61	0.95	B17P	0.40	0.49	0.91
B18	0.48	11.85	16.82	B18P	0.66	9.20	13.89
B22	0.46	0.11	0.15	B22P	0.33	0.02	0.05
B24	0.42	6.39	9.18	B24aP	0.55	4.46	7.15
				B24bP	0.62	0.98	1.42
D4	0.55	2.99	4.13	D4P	0.50	5.57	9.24
D8a	0.55	0.86	1.19	D8aP	0.58	5.12	8.36
D8b	0.35	0.28	0.42	D8bP	0.66	1.14	1.64
D9	0.54	9.71	13.56	D9P	0.68	8.43	12.37

As shown in Table 1, the calculated C values and runoff produced by a 25-year storm event for the Project match or are less than the designed values in the Report and the Project conforms to the Report. Basins D4, D8a & D8b, as proposed, have a higher runoff value than shown in the Report, pond volume and outlet structure release rates have been analyzed, and any necessary modifications can be found in Appendix B.

It was discovered that the Report inaccurately modeled the existing inlets in a sump condition, when they are on-grade inlets. Since on-grade inlets have a much lower interception capacity, much of the runoff originally assumed to be totally captured by the inlets will bypass its respective basin inlet and continue downstream to other inlets. All of the existing inlets have been correctly modeled with bypass flows accounted for, inlet calculations with bypass accounted for can be found in Appendix C.

As a result of the inlets being modeled as on-grade inlets, bypass flows from upstream inlets flow downstream to other inlets which convey flows to different ponds than originally planned. This makes it impossible to model and verify pond sizing based on true tributary area and imperviousness. The method used to most accurately model the flows going to each pond was to hold the imperviousness percentage consistent with what was shown in the Report and multiply the area of the major basins by a ratio of Actual Q₁₀₀ to the Q₁₀₀ per the Report going into each pond. Pond Calculations for Ponds 1A, 1B & 1D can be found in Appendix C. See below for pond area conversion example:

Pond 1A:

Basin A Area: 7.96 AC

Q₁₀₀ entering Pond per Report: 20.80 CFS

Q₁₀₀ entering Pond per calculations: 22.43 CFS

Ratio: 22.43 CFS:20.80 CFS = 1.078

Basin A Area as modeled in pond verification calculations: 7.96 AC * 1.078 = 8.58 AC

Project Name: Haymeadow Filling 2
Project No: 240804
Date: 11/18/24
Revised: 01/00/00

STANDARD FORM SF-2
Post-Development
Rational Method Procedure

Designed By: CLH
Checked By: RCP
Design Storm: 25 YR

39.89
30.56

BASIN (S)	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET/INLET				STORM SEWER PIPE		TRAVEL TIME			CARRYOVER FLOWS				REMARKS			
		AREA (AC)	RUNOFF COEFF	Tc (min)	C x A (AC)	I (IN/HR)	DIRECT RUNOFF, Q (CFS)	Tc (MAX)	Σ(C x A) (AC)	I (IN/HR)	TOTAL RUNOFF, Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	INLET DESIGN FLOW (CFS)	STREET OR INLET INTERCEPTION (CFS)	CARRYOVER (CFS)	DESIGN FLOW (CFS)	PIPE SLOPE (%)	PIPE SIZE (IN)	Q@FULL (CFS)	LENGTH (FT)	VELOCITY (FPS)	Tc (min)	BYPASS RUNOFF, Q (CFS)		LENGTH (FT)	SLOPE (%)	VELOCITY (FPS)
B17P		0.61	0.46	11.66	0.24	3.24	0.78	1.66	0.40					1.3	0.6		1.00	24	29.41	94	1.5	1.05						
B18P		4.75	0.66	15.71	3.13	2.70	8.44	1.00	3.13				8.44	0			1.00	18	13.66	44	1.5	0.49						
B22P		0.03	0.33	10.53	0.01	3.43	0.03	1.00	0.01				0.03	0.00			2.00	18	19.31	46	2.1	0.36						
B22-2		1.72	0.47	14.96	0.81	2.79	2.26	1.00	0.81				2.26	0.00			2.00	18	19.31	144	2.1	1.13						
B24aP		4.27	0.55	15.40	2.34	2.73	6.40	0.30	0.69				1.9	4.5			4.71	18	29.64	60	3.3	0.31						
B24bP		1.71	0.62	11.17	1.06	3.32	3.51	0.43	0.45				1.5	2.1			1.00	18	13.66	22	1.5	0.24						
D1P		2.59	0.60	15.57	1.54	2.72	4.18	0.48	0.74				2	2.1			2.85	24	49.65	64	2.5	0.42						
D4P		4.72	0.50	18.92	2.35	2.37	5.57	0.36	0.84				2.00	3.80			2.16	18	20.07	86	2.2	0.65						
D8aP		2.88	0.58	17.97	1.67	2.46	4.11	0.49	0.81				2.00	2.10			3.87	18	26.86	26	3.0	0.15						
D8bP		4.11	0.66	11.81	0.73	3.22	2.36	0.64	0.47				1.50	0.90			1.00	18	13.66	22	1.5	0.25						
D9P		5.72	0.68	14.42	3.90	2.85	11.13	1.00	3.90				11.13	0.00			3.13	18	24.16	34	2.7	0.21						
DX		1.11	0.69	13.94	0.77	2.91	2.23	0.00	0.00																			
E1		0.29	0.25	10.33	0.10	3.36	0.35																					
F2		0.09	0.68	5.01	0.06	4.74	0.30																					
F3		0.84	0.67	13.93	0.58	3.20	1.81																					
T4		1.37	0.55	13.87	0.76	2.92	2.23																					
T5		0.26	0.33	17.49	0.09	2.51	0.22																					
U3		0.26	0.55	8.10	0.14	3.92	0.55																					
U4		0.41	0.66	7.47	0.27	4.06	1.09																					
U5		1.03	0.33	12.34	0.34	3.14	1.07																					
V1		0.41	0.63	10.00	0.26	3.53	0.90																					
V2		0.95	0.67	15.64	0.64	2.71	1.73																					
W3		0.39	0.62	12.04	0.24	3.18	0.77																					
WC1		2.13	0.37	14.31	0.79	2.87	2.27																					
WC2		1.25	0.38	12.50	0.47	3.11	1.48																					
CS1		0.29	0.38	10.85	0.11	3.38	0.37																					
CS2		2.48	0.33	8.98	0.82	3.73	3.06																					
CS3		0.47	0.38	5.00	0.14	4.75	0.67																					
CS4		0.16	0.33	5.00	0.05	3.00	0.16																					
Y B24aP & B24bP	1							15.40	1.15	2.73	3.1																	
Y DP1 & B23	2							15.71	1.70	2.70	4.6																	
Y B22P & B22-2	3							16.09	0.82	2.66	2.2																	
Y B21, DP2 & DP3	4							16.45	2.98	2.62	7.8																	
Y B20 & DP4	5							16.83	3.35	2.58	8.6																	
Y B19 & DP5	6							17.08	3.72	2.55	9.5																	
Y B17, B18 & DP6	7							18.08	7.25	2.45	17.8																	
Y B16 & DP7	8							19.13	7.47	2.36	17.6																	
Y B13, B14 & B15	9							11.24	0.74	3.31	2.5																	
Y B11, B12, DP8 & DP9	10							19.42	8.48	2.33	19.8																	
Y B10 & DP10	11							19.65	8.79	2.31	20.3																	
Y B9 & DP11	12							19.95	8.91	2.29	20.4																	
Y B8, B7 & DP12	13							20.79	9.40	2.22	20.8																	
Y B6 & DP13	14							21.22	9.89	2.18	21.6																	
Y B5, B4 & DP14	15							22.25	10.39	2.11	21.9																	
Y C7 & C6	16							10.20	0.71	3.49	2.5																	
Y C5 & DP16	17							10.45	0.89	3.45	3.1																	
Y C4 & DP17	18							11.40	0.98	3.29	3.2																	
Y C3 & DP18	19							11.82	1.23	3.22	4.0																	
Y C2 & DP19	20							12.48	1.26	3.12	3.9																	
Y C1 & DP20	21							12.81	2.03	3.07	6.2																	
Y C0 & DP21	22							13.80	2.71	2.93	7.9																	
Y B2, DP15 & DP22	23							22.78	13.80	2.07	28.6																	
Y DP9 & D8aP	24							14.63	4.37	2.83	12.3																	
Y D8aP & DP24	25							17.97	5.18	2.46	12.8																	
Y D7 & DP25	26							18.11	5.61	2.45	13.7																	
Y D6, D5 & DP26	27							18.92	5.83	2.37	13.8																	
Y D4P & DP27	28							19.27	6.68	2.34	15.6																	
Y D3, D2 & DP28	29							19.92	7.52	2.29	17.2																	
Y D1P & DP29	30							20.51	8.25	2.24	18.5																	

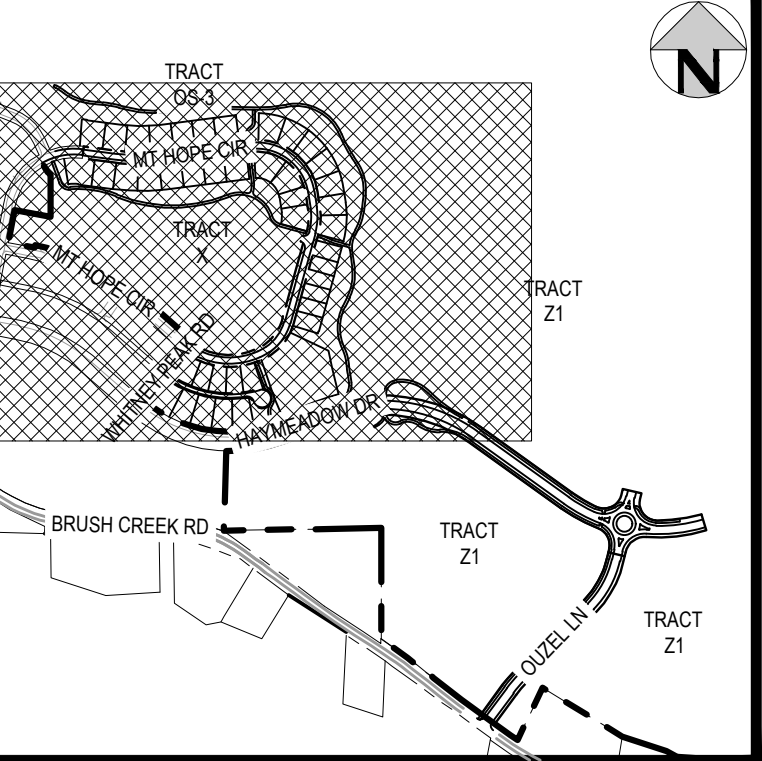
Project Name: Haymeadow Filing 2
 Project No: 240804
 Date: 11/18/24
 Revised: 01/00/00

STANDARD FORM SF-2
Post-Development
Rational Method Procedure

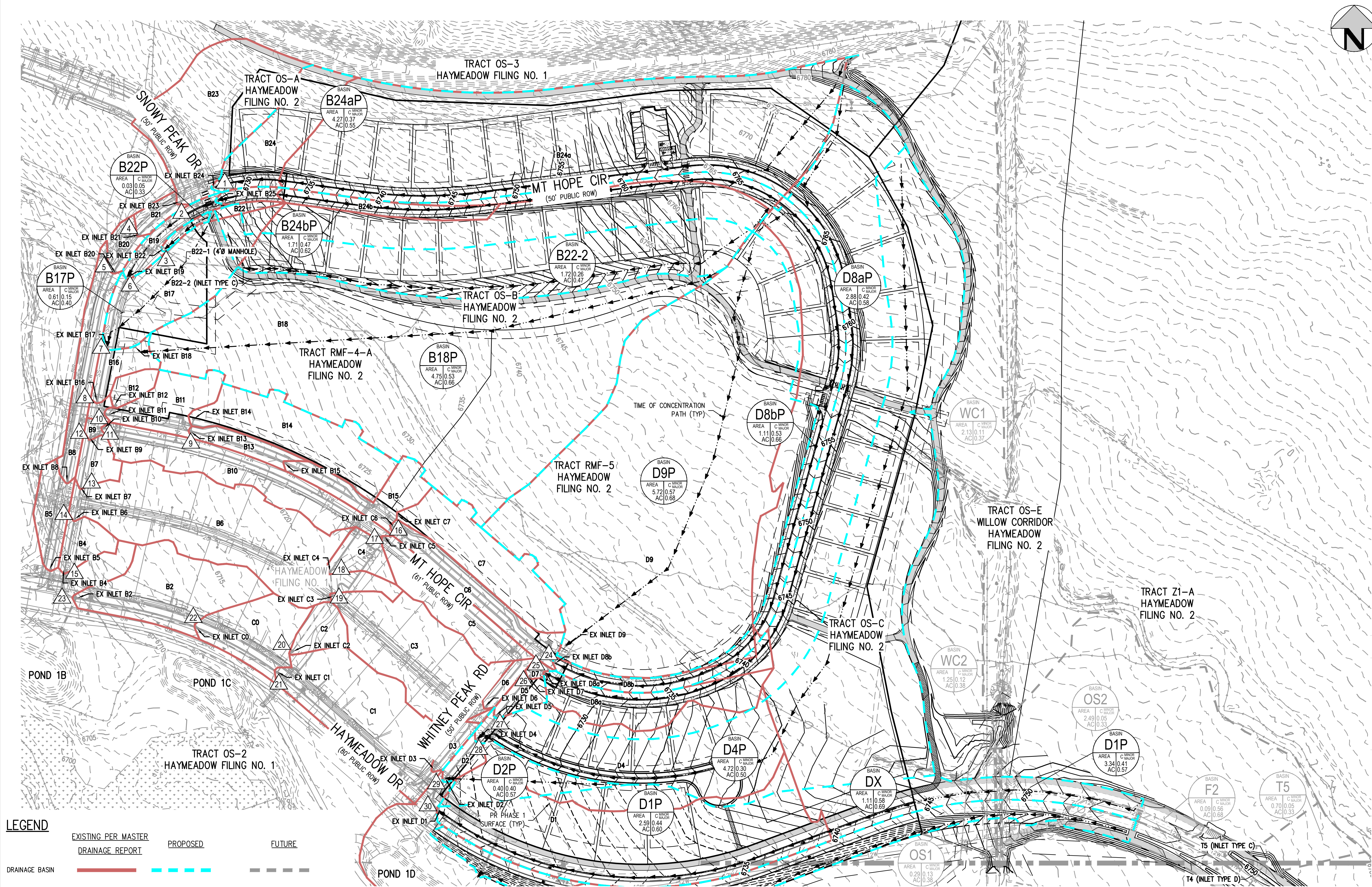
Designed By: CLH
 Checked By: RCP
 Design Storm: 100 YR

BASIN (s)	DESIGN POINT	DIRECT RUNOFF					TOTAL RUNOFF			STREET/INLET		STORM SEWER PIPE		TRAVEL TIME		CARRYOVER FLOWS				REMARKS					
		AREA (AC)	RUNOFF COEFF	Tc (min)	C x A (AC)	I (IN/HR)	Tc (MAX)	Σ(C x A) (AC)	I (IN/HR)	TOTAL RUNOFF, Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	INLET DESIGN FLOW (CFS)	DESIGN FLOW (CFS)	PIPE SLOPE (%)	PIPE SIZE (IN)	QFULL (CFS)	LENGTH (FT)	VELOCITY (FPS)		Tt (min)	BYPASS RUNOFF, Q (CFS)	LENGTH (FT)	SLOPE (%)	VELOCITY (FPS)
B17P		0.61	0.54	11.66	0.33	4.36	1.43	1.40	0.46					1.00	24	29.41	94	1.5	1.05						
B18P		4.75	0.73	15.71	3.48	3.63	12.62	1.00	3.48			12.62	0	1.00	18	13.66	44	1.5	0.49						
B22P		0.03	0.49	10.53	0.01	4.62	0.07	1.00	0.01			0.07	0.00	2.00	18	19.31	46	2.1	0.36						
B22-2		1.72	0.60	14.96	1.03	3.74	3.84	1.00	1.03			3.84	0.00	2.00	18	19.31	##	2.1	1.13						
B24aP		4.27	0.65	15.40	2.78	3.67	10.23	0.21	0.57			2.10	8.20	4.71	18	29.64	60	3.3	0.31						
B24bP		1.71	0.70	11.17	1.20	4.47	5.97	0.32	0.38			1.7	3.6	1.00	18	13.66	22	1.5	0.24						
D1P		2.59	0.69	15.57	1.78	3.65	6.48	0.39	0.69			2.5	3.9	2.85	24	49.65	64	2.5	0.42						
D4P		4.72	0.61	18.92	2.90	3.19	9.24	0.27	0.78			2.50	7.70	2.16	18	20.07	86	2.2	0.65						
D8aP		2.88	0.67	17.97	1.94	3.31	6.42	0.40	0.79			2.60	3.90	3.87	18	26.86	28	3.0	0.15						
D8bP		1.11	0.73	11.81	0.81	4.33	9.51	0.54	0.44			1.90	1.60	1.00	18	13.66	22	1.5	0.25						
D9P		5.72	0.75	14.42	4.29	3.83	16.44	1.00	4.29			16.44	0.00	3.13	18	24.16	34	2.7	0.21						
DX		1.11	0.76	13.94	0.84	3.92	3.29	0.00	0.00																
E1		0.28	0.51	10.93	0.15	4.52	0.87																		
E2		0.06	0.75	5.01	0.07	5.39	0.44																		
F3		0.84	0.74	11.93	0.62	4.30	2.68																		
F4		1.37	0.68	13.87	0.90	3.93	3.55																		
F5		0.26	0.49	17.49	0.13	3.97	0.44																		
G3		0.26	0.65	8.10	0.17	5.27	0.88																		
G4		0.41	0.73	7.47	0.30	5.47	1.64																		
G5		1.03	0.49	12.39	0.51	4.22	2.14																		
G6		0.41	0.71	10.00	0.29	4.75	1.37																		
G7		0.95	0.74	15.64	0.70	3.64	2.56																		
G8		0.39	0.71	12.04	0.28	4.28	1.18																		
WC1		2.13	0.52	14.31	1.11	3.95	4.29																		
WC2		1.26	0.53	12.50	0.66	4.19	2.77																		
OS1		0.29	0.53	10.85	0.15	4.54	0.69																		
OS2		2.49	0.49	8.98	1.22	5.01	6.14																		
OS3		0.37	0.53	8.00	0.19	6.39	1.25																		
OS4		0.18	0.49	5.00	0.08	6.39	0.51																		
Σ B24aP & B24bP	1							15.40	0.95	3.67	3.5														
Σ DP1 & B23	2							15.71	1.46	3.63	5.3														
Σ B22P & B22-2	3							16.09	1.04	3.57	3.7														
Σ B21, DP2 & DP3	4							16.45	2.99	3.52	10.5														
Σ B20 & DP4	5							16.83	3.43	3.46	11.9														
Σ B19 & DP5	6							17.08	3.79	3.43	13.0														
Σ B17, B18 & DP6	7							18.08	7.73	3.29	25.4														
Σ B16 & DP7	8							19.13	6.07	3.16	25.5														
Σ B13, B14 & B15	9							11.24	0.87	4.45	3.9														
Σ B11, B12, DP8 & DP9	10							19.42	9.45	3.13	29.6														
Σ B10 & DP10	11							19.65	9.75	3.10	30.2														
Σ B9 & DP11	12							19.95	9.89	3.07	30.3														
Σ B8, B7 & DP12	13							20.79	10.57	2.98	31.5														
Σ B6 & DP13	14							21.22	11.03	2.93	32.3														
Σ B5, B4 & DP14	15							22.25	11.78	2.83	33.2														
Σ C7 & C6	16							10.20	0.76	4.70	3.6														
Σ C5 & DP16	17							10.45	0.96	4.64	4.4														
Σ C4 & DP17	18							11.40	1.05	4.42	4.6														
Σ C3 & DP19	19							11.82	1.29	4.33	5.6														
Σ C2 & DP19	20							12.48	1.34	4.19	5.6														
Σ C1 & DP20	21							12.81	2.16	4.13	8.9														
Σ C0 & DP21	22							13.80	2.94	3.94	11.6														
Σ B2, DP15 & DP22	23							22.78	15.52	2.78	43.4														
Σ DP9 & D8bP	24							14.63	4.73	3.80	18.0														
Σ D8aP & DP24	25							17.97	5.51	3.31	18.2														
Σ D7 & DP25	26							18.11	6.02	3.29	19.8														
Σ D6, D5 & DP26	27							18.92	6.36	3.19	20.3														
Σ D4P & DP27	28							19.27	7.14	3.15	22.5														
Σ D3, D2 & DP28	29							19.92	7.76	3.07	23.8														
Σ D1P & DP29	30							20.51	8.45	3.01	25.4														

% FULL



KEY MAP
SCALE: 1" = 800'



LEGEND

EXISTING PER MASTER DRAINAGE REPORT PROPOSED FUTURE

DRAINAGE BASIN ——— - - - - -

BASIN LABEL D#

BASIN DESIGNATION OS 1

BASIN SIZE IN ACRES 1.23 0.45 0.67

BASIN DESIGN POINT 1

X-YR RATIONAL C COEFFICIENT

XX-YR RATIONAL C COEFFICIENT



Appendix C – Calculations

Project Name: Haymeadow RMF 4 & 5

Composite C-Value Computations

Post-Development

Project No: 2504026

Date: 08/01/25

Revised: -

Design by: ORR

Checked by: RCP

Hydrologic Soil Group: C

BASIN	TOTAL AREA (ACRES)	ROOFS (90%)	DRIVES & WALKS (90%)	GARAGE & STREETS (100%)	MULTIFAMILY (75%)	SINGLE FAMILY (45%)	LANDSCAPE AREA (2%)	PERCENT IMPERVIOUS	C _s =	C ₂₅ =	C ₁₀₀ =
FILING 1 MASTER DRAINAGE REPORT - STORM LINE B											
B1	2.92	0.42	0.07	0.33	0.00	0.10	2.75	30%	0.28	0.49	0.61
B2	0.66	0.09	0.06	0.15	0.00	0.00	0.36	44%	0.39	0.56	0.66
B13P	0.71	0.08	0.02	0.09	0.00	0.00	0.53	26%	0.25	0.46	0.59
B16P	0.21	0.05	0.01	0.03	0.00	0.00	0.12	41%	0.38	0.55	0.65
B17P2	0.58	0.00	0.04	0.04	0.00	0.00	0.51	14%	0.15	0.40	0.54
B18P2	0.12	0.03	0.00	0.00	0.00	0.00	0.09	22%	0.21	0.44	0.57
FILING 1 MASTER DRAINAGE REPORT - STORM LINE C											
C1	1.46	0.35	0.08	0.12	0.00	0.00	0.91	36%	0.33	0.52	0.63
C2	1.04	0.15	0.10	0.36	0.00	0.00	0.43	57%	0.50	0.64	0.72
C6P	0.63	0.10	0.04	0.20	0.00	0.00	0.29	52%	0.46	0.61	0.70
FILING 1 MASTER DRAINAGE REPORT - STORM LINE D											
D1	0.53	0.11	0.04	0.17	0.00	0.00	0.20	60%	0.52	0.65	0.73
D2	0.82	0.07	0.07	0.17	0.00	0.19	0.32	48%	0.43	0.59	0.68
D3	0.32	0.12	0.01	0.00	0.00	0.00	0.19	38%	0.35	0.53	0.64
D4	0.46	0.11	0.06	0.15	0.00	0.00	0.13	68%	0.59	0.70	0.76
D5	0.27	0.07	0.01	0.00	0.00	0.00	0.19	28%	0.26	0.48	0.60
D6	0.37	0.07	0.08	0.16	0.00	0.00	0.06	80%	0.69	0.77	0.81
D7	0.46	0.02	0.01	0.22	0.00	0.00	0.21	54%	0.48	0.62	0.71
D8	0.58	0.09	0.01	0.13	0.00	0.00	0.35	39%	0.35	0.53	0.64
D8bP2	1.06	0.00	0.13	0.38	0.00	0.37	0.17	63%	0.55	0.67	0.74
D9P2	0.77	0.24	0.00	0.00	0.00	0.00	0.53	29%	0.28	0.48	0.60

Project Name: Haymeadow RMF 4 & 5

Project No: 2504026

Date: 8/1/2025

Revised: -

STANDARD FORM SF-2 TIME OF CONCENTRATION

Designed By: ORR

Checked By: RCP

SUB-BASIN DATA			INITIAL/OVERLAND TIME (Ti)			TRAVEL TIME (Ti)					Tc CHECK (URBANIZED BASINS)			FINAL
BASIN	AREA (AC)	C _s	LENGTH (FT)	SLOPE %	Ti (MIN)	LENGTH (FT)	SLOPE %	C _v	VELOCITY (FPS)	Tt (MIN)	COMPOS. Tc (MIN)	TOTAL LENGTH	Tc = (L/180) + 10 (MIN)	Tc (MIN)
FILING 1 MASTER DRAINAGE REPORT - STORM LINE B														
B1	2.92	0.28	167	4.7	11.62	850	3.0	20	3.46	4.09	15.71	1,017	15.65	15.65
B2	0.66	0.39	65	6.5	5.58	308	2.0	20	2.83	1.81	7.39	373	12.07	7.39
B13P	0.71	0.25	71	4.9	7.73	331	3.0	20	3.46	1.59	9.32	402	12.23	9.32
B16P	0.21	0.38	68	3.6	7.15	70	2.0	7	0.99	1.18	8.33	138	10.77	8.33
B17P2	0.58	0.15	298	5.7	16.88	1	2.0	7	0.99	0.02	16.89	299	11.66	11.66
B18P2	0.12	0.21	149	3.6	12.96	1	2.0	7	0.99	0.02	12.98	150	10.83	10.83
FILING 1 MASTER DRAINAGE REPORT - STORM LINE C														
C1	1.46	0.33	382	4.3	17.01	39	3.1	20	3.52	0.18	17.19	421	12.34	12.34
C2	1.04	0.50	211	5.9	8.85	267	3.9	20	3.93	1.13	9.99	478	12.66	9.99
C6P	0.63	0.46	71	4.6	5.94	181	2.1	20	2.93	1.03	6.97	252	11.40	6.97
FILING 1 MASTER DRAINAGE REPORT - STORM LINE D														
D1	0.53	0.52	121	5.3	6.69	167	3.8	20	3.88	0.72	7.41	288	11.60	7.41
D2	0.82	0.43	216	4.7	10.83	105	2.6	20	3.19	0.55	11.38	321	11.78	11.38
D3	0.32	0.35	48	6.0	5.26	101	2.0	7	0.99	1.70	6.96	149	10.83	6.96
D4	0.46	0.59	47	2.1	4.96	119	3.5	20	3.74	0.53	5.49	166	10.92	5.49
D5	0.27	0.26	54	3.9	7.18	70	2.0	7	0.99	1.18	8.35	124	10.69	8.35
D6	0.37	0.69	28	2.9	2.79	62	3.2	20	3.58	0.29	5.00	90	10.50	5.00
D7	0.46	0.48	80	5.0	5.95	118	4.1	20	4.05	0.49	6.43	198	11.10	6.43
D8	0.58	0.35	241	4.1	13.28	25	4.5	20	4.24	0.10	13.37	266	11.48	11.48
D8bP2	1.06	0.55	63	2.0	6.31	1123	3.2	20	3.55	5.27	11.58	1,186	16.59	11.58
D9P2	0.77	0.28	49	2.0	8.39	700	2.0	7	0.99	11.79	20.18	749	14.16	14.16

Project Name: Haymeadow RMF 4 & 5
Project No: 2504026
Date: 08/01/25
Revised: -

STANDARD FORM SF-2
Post-Development
Rational Method Procedure

Designed By: ORR
Checked By: RCP
Design Storm: 5 YR

BASIN (s)	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET/INLET				STORM SEWER PIPE				TRAVEL TIME		CARRYOVER FLOWS				REMARKS		
		AREA (AC)	RUNOFF COEFF	T _c (min)	C x A (AC)	I (IN/HR)	DIRECT RUNOFF, Q (CFS)	T _c (min)	Σ(C x A) (AC)	I (IN/HR)	TOTAL RUNOFF, Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	INLET DESIGN FLOW (CFS)	STREET OR INLET INTERCEPTION (CFS)	CARRYOVER (CFS)	DESIGN FLOW (CFS)	PIPE SLOPE (%)	PIPE SIZE (IN)	QF ALL (CFS)	LENGTH (FT)	VELOCITY (FPS)	T _t (min)	BYPASS RUNOFF, Q (CFS)	LENGTH (FT)		SLOPE (%)	VELOCITY (FPS)
FILING 1 MASTER DRAINAGE REPORT - STORM LINE B																												
B1	1	2.92	0.28	15.65	0.82	1.63	1.3									1.3	2.0	18	19.3	190	2.1	1.49						direct runoff to inlet at DP1
B2	2	0.66	0.39	7.39	0.26	2.45	0.6									0.6	0.5	18	9.7	190	1.1	2.99						direct runoff to inlet at DP 2
B13P Σ DP 1 + DP 2 + DP 3	3	0.71	0.25	9.32	0.18	2.19	0.4	17.14	1.25	1.53	1.9																	direct runoff to inlet at DP 3 total flow to Master Report Design Point 1:
B16P		0.21	0.38	8.33	0.08	2.31	0.2																					total flow to Master Report Inlet B12
B17P2		0.58	0.15	11.66	0.09	1.94	0.2																					total flow to Master Report Inlet B17
B18P2		0.12	0.21	10.83	0.03	2.02	0.1																					total flow to Master Report Inlet B18
FILING 1 MASTER DRAINAGE REPORT - STORM LINE C																												
C1	4	1.46	0.33	12.34	0.48	1.88	0.9									0.9	2.8	18	22.9	140	2.5	0.93						direct runoff to inlet at DP 4
C2 Σ DP 4 + DP 5	5 6	1.04	0.50	9.99	0.52	2.11	1.1	13.27	1.00	1.80	1.8					1.1	1.0	18	13.7	18	1.5	0.20						direct runoff to inlet at DP 5 combined flow in storm sewer at DP 4
C6P Σ DP 6 + DP 7	7	0.63	0.46	6.97	0.29	2.51	0.7	13.49	1.29	1.78	2.3																	direct runoff to inlet at DP 7 combined flow in storm sewer at DP 7, total flow to Master Report DP 18
FILING 1 MASTER DRAINAGE REPORT - STORM LINE D																												
D1	8	0.53	0.52	7.41	0.28	2.44	0.7									0.7	0.7	18	11.4	171	1.3	2.27						direct runoff to inlet at DP 8
D3 Σ DP 8 + DP 9	9 10	0.32	0.35	6.96	0.11	2.51	0.3	9.68	0.39	2.15	0.8					0.3	5.0	18	30.5	36	3.4	0.18						direct runoff to inlet at DP 9 combined flow in storm sewer at DP 11
D2 Σ DP 11 + DP 10	11 12	0.82	0.43	11.38	0.35	1.97	0.7	11.42	0.74	1.97	1.4					0.7	10.0	18	43.2	10	4.7	0.04						direct runoff to inlet at DP 11 combined flow in storm sewer at DP 1:
D4 Σ DP 4 + DP 12	13	0.46	0.59	5.49	0.27	2.76	0.8	12.17	1.01	1.90	1.9					1.9	1.6	18	17.3	185	1.9	1.63						direct runoff to inlet at DP 13 combined flow in storm sewer at DP 1:
D5 Σ DP 13 + DP 14	14 15	0.27	0.26	8.35	0.07	2.31	0.2	13.80	1.08	1.76	1.9					0.2	4.6	18	29.3	58	3.2	0.30						direct runoff to inlet at DP 14 combined flow in storm sewer at DP 1:
D6 Σ DP 15 + D6	16	0.37	0.69	5.00	0.25	2.86	0.7	13.91	1.33	1.75	2.3					2.3	0.6	18	10.6	149	1.2	2.14						direct runoff to inlet at DP 16 combined flow in storm sewer at DP 11
D7 Σ DP 16 + DP 17	17 18	0.46	0.48	6.43	0.22	2.60	0.6	16.05	1.55	1.60	2.5					0.6	4.5	18	29.0	81	3.2	0.42						direct runoff to inlet at DP 17 combined flow in storm sewer at DP 11
D8 Σ DP 8 + DP 18	19	0.58	0.35	11.48	0.20	1.96	0.4	16.92	1.76	1.55	2.7					2.7	0.5	18	9.7	44	1.1	0.69						direct runoff to inlet at DP 19 combined flow in storm sewer at DP 11
D9P2	33	0.77	0.28	14.16	0.21	1.73	0.4	17.61	1.97	1.51	3.0																	direct runoff to inlet at DP 19 total flow to Master Report DP 33
D8bP2		1.06	0.55	11.58	0.59	1.95	1.1																					

Project Name: Haymeadow RMF 4 & 5
Project No: 2504026
Date: 08/01/25
Revised: -

STANDARD FORM SF-2
Post-Development
Rational Method Procedure

Designed By: ORR
Checked By: RCP
Design Storm: 25 YR

BASIN (s)	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET/INLET				STORM SEWER PIPE				TRAVEL TIME		CARRYOVER FLOWS				REMARKS			
		AREA (AC)	RUNOFF COEFF	Tc (min)	C x A (AC)	I (IN/HR)	DIRECT RUNOFF, Q (CFS)	Tc (min)	Σ(C x A) (AC)	I (IN/HR)	TOTAL RUNOFF, Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	INLET DESIGN FLOW (CFS)	STREET OR INLET INTERCEPTION (CFS)	CARRYOVER (CFS)	DESIGN FLOW (CFS)	PIPE SLOPE (%)	PIPE SIZE (IN)	QFULL (CFS)	LENGTH (FT)	VELOCITY (FPS)	Tt (min)	BYPASS RUNOFF, Q (CFS)	LENGTH (FT)		SLOPE (%)	VELOCITY (FPS)	Tt (min)
FILING 1 MASTER DRAINAGE REPORT - STORM LINE B																													
B1	1	2.92	0.49	15.65	1.42	2.71	3.8									3.8	2.0	18	19.3	190	2.1	1.49							direct runoff to inlet at DP1
B2	2	0.66	0.56	7.39	0.37	4.08	1.5									1.5	0.5	18	9.7	190	1.1	2.99							direct runoff to inlet at DP 2
B13P Σ DP 1 + DP 2 + DP 3	3	0.71	0.46	9.32	0.33	3.66	1.2	17.14	2.12	2.54	5.4																		direct runoff to inlet at DP 3 total flow to Master Report Design Point 1:
B16P		0.21	0.55	8.33	0.12	3.86	0.4																						total flow to Master Report Inlet B12
B17P2		0.58	0.40	11.66	0.23	3.24	0.8																						total flow to Master Report Inlet B17
B18P2		0.12	0.44	10.83	0.05	3.38	0.2																						total flow to Master Report Inlet B18
FILING 1 MASTER DRAINAGE REPORT - STORM LINE C																													
C1	4	1.46	0.52	12.34	0.76	3.14	2.4									2.4	2.8	18	22.9	140	2.5	0.93							direct runoff to inlet at DP 4
C2 Σ DP 4 + DP 5	5 6	1.04	0.64	9.99	0.66	3.53	2.3	13.27	1.42	3.00	4.3					2.3	1.0	18	13.7	18	1.5	0.20							direct runoff to inlet at DP 5 combined flow in storm sewer at DP 4
C6P Σ DP 6 + DP 7	7	0.63	0.61	6.97	0.38	4.19	1.6	13.49	1.80	2.97	5.4																		direct runoff to inlet at DP 7 combined flow in storm sewer at DP 7, total flow to Master Report DP 18
FILING 1 MASTER DRAINAGE REPORT - STORM LINE D																													
D1	8	0.53	0.65	7.41	0.34	4.08	1.4									1.4	0.7	18	11.4	171	1.3	2.27							direct runoff to inlet at DP 8
D3 Σ DP 8 + DP 9	9 10	0.32	0.53	6.96	0.17	4.19	0.7	9.68	0.51	3.59	1.8					0.7	5.0	18	30.5	36	3.4	0.18							direct runoff to inlet at DP 9 combined flow in storm sewer at DP 11
D2 Σ DP 11 + DP 10	11 12	0.82	0.59	11.38	0.48	3.29	1.6	11.42	0.99	3.28	3.3					1.6	10.0	18	43.2	10	4.7	0.04							direct runoff to inlet at DP 11 combined flow in storm sewer at DP 1:
D4 Σ D4 + DP 12	13	0.46	0.70	5.49	0.32	4.59	1.5	12.17	1.31	3.16	4.2					4.2	1.6	18	17.3	185	1.9	1.63							direct runoff to inlet at DP 13 combined flow in storm sewer at DP 1:
D5 Σ DP 13 + DP 14	14 15	0.27	0.48	8.35	0.13	3.86	0.5	13.80	1.44	2.93	4.2					0.5	4.6	18	29.3	58	3.2	0.30							direct runoff to inlet at DP 14 combined flow in storm sewer at DP 1:
D6 Σ DP 15 + D6	16	0.37	0.77	5.00	0.28	4.75	1.3	13.91	1.73	2.92	5.0					5.0	0.6	18	10.6	149	1.2	2.14							direct runoff to inlet at DP 16 combined flow in storm sewer at DP 11
D7 Σ DP 16 + DP 17	17 18	0.46	0.62	6.43	0.29	4.33	1.2	16.05	2.01	2.66	5.4					1.2	4.5	18	29.0	81	3.2	0.42							direct runoff to inlet at DP 17 combined flow in storm sewer at DP 11
D8 Σ D8 + DP 18	19	0.58	0.53	11.48	0.31	3.27	1.0	16.92	2.32	2.57	6.0					6.0	0.5	18	9.7	44	1.1	0.69							direct runoff to inlet at DP 19 combined flow in storm sewer at DP 11
D9P2	33	0.77	0.48	14.16	0.37	2.89	1.1	17.61	2.69	2.50	6.7																		direct runoff to inlet at DP 19 total flow to Master Report DP 33
D8bP2		1.06	0.67	11.58	0.71	3.26	2.3																						

Project Name: Haymeadow RMF 4 & 5
Project No: 2504026
Date: 08/01/25
Revised: -

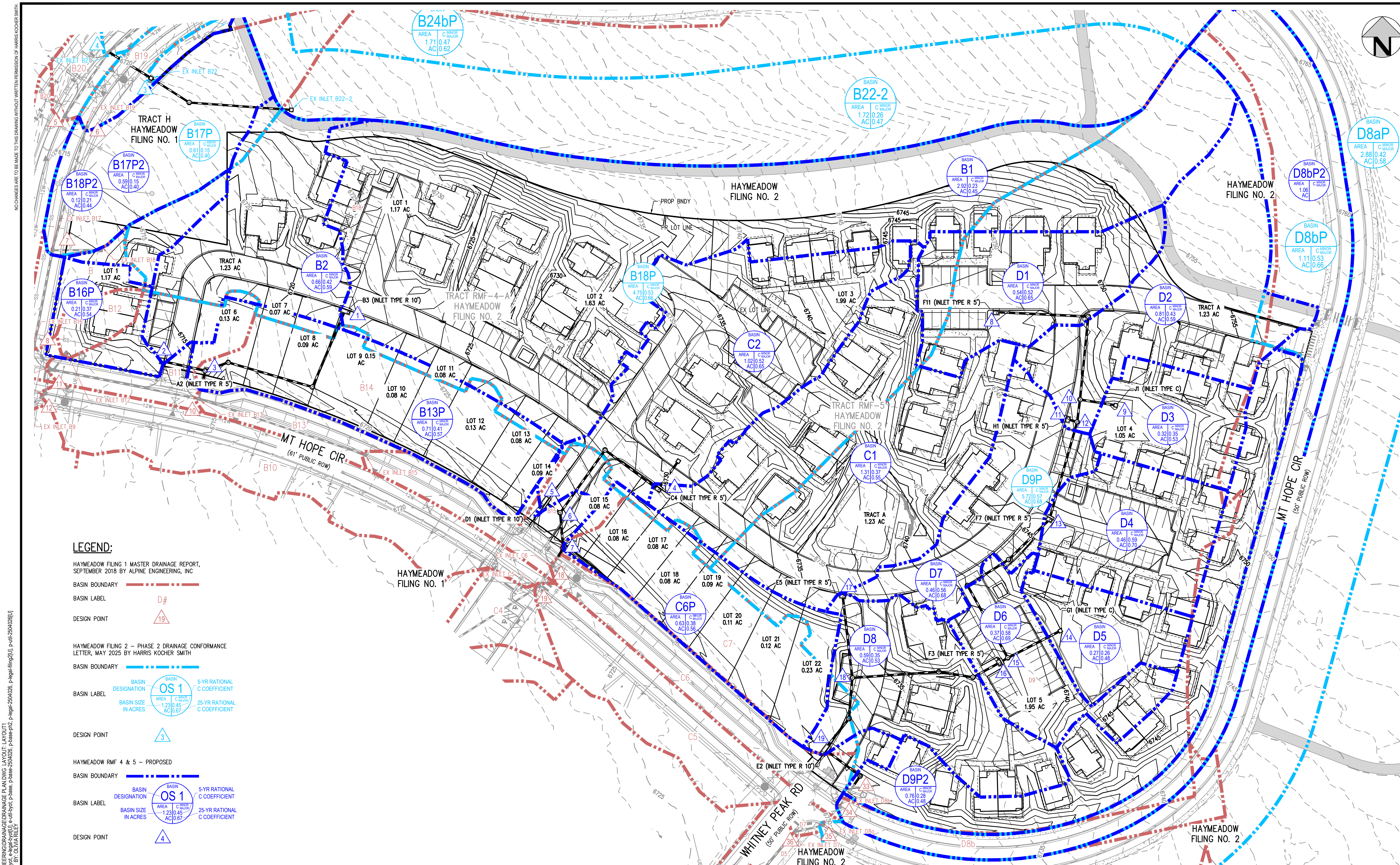
STANDARD FORM SF-2
Post-Development
Rational Method Procedure

Designed By: ORR
Checked By: RCP
Design Storm: 100 YR

BASIN (s)	DESIGN POINT	DIRECT RUNOFF						TOTAL RUNOFF				STREET/INLET				STORM SEWER PIPE			TRAVEL TIME		CARRYOVER FLOWS				REMARKS				
		AREA (AC)	RUNOFF COEFF	Tc (min)	C x A (AC)	I (IN/HR)	DIRECT RUNOFF, Q (CFS)	Tc (min)	Σ(C x A) (AC)	I (IN/HR)	TOTAL RUNOFF, Q (CFS)	SLOPE (%)	STREET FLOW (CFS)	INLET DESIGN FLOW (CFS)	STREET OR INLET INTERCEPTION (CFS)	CARRYOVER (CFS)	DESIGN FLOW (CFS)	PIPE SLOPE (%)	PIPE SIZE (IN)	QFULL (CFS)	LENGTH (FT)	VELOCITY (FPS)	Tt (min)	BYPASS RUNOFF, Q (CFS)		LENGTH (FT)	SLOPE (%)	VELOCITY (FPS)	Tt (min)
FILING 1 MASTER DRAINAGE REPORT - STORM LINE B																													
B1	1	2.92	0.61	15.65	1.77	3.63	6.4									6.4	2.0	18	19.3	190	2.1	1.49						direct runoff to inlet at DP1	
B2	2	0.66	0.66	7.39	0.44	5.49	2.4									2.4	0.5	18	9.7	190	1.1	2.99						direct runoff to inlet at DP 2	
B13P Σ DP 1 + DP 2 + DP 3	3	0.71	0.59	9.32	0.42	4.92	2.1	17.14	2.63	3.42	9.0																	direct runoff to inlet at DP 3 total flow to Master Report Design Point 1:	
B16P		0.21	0.65	8.33	0.14	5.20	0.7																					total flow to Master Report Inlet B12	
B17P2		0.58	0.54	11.66	0.32	4.36	1.4																					total flow to Master Report Inlet B17	
B18P2		0.12	0.57	10.83	0.07	4.54	0.3																					total flow to Master Report Inlet B18	
FILING 1 MASTER DRAINAGE REPORT - STORM LINE C																													
C1	4	1.46	0.63	12.34	0.92	4.22	3.9									3.9	2.8	18	22.9	140	2.5	0.93						direct runoff to inlet at DP 4	
C2 Σ DP 4 + DP 5	5 6	1.04	0.72	9.99	0.75	4.75	3.5	13.27	1.67	4.04	6.7					6.7	3.0	18	23.7	35	2.6	0.22						direct runoff to inlet at DP 5 combined flow in storm sewer at DP 4	
C6P Σ DP 6 + DP 7	7	0.63	0.70	6.97	0.44	5.63	2.5	13.49	2.11	4.00	8.4																	direct runoff to inlet at DP 7 combined flow in storm sewer at DP 7, total flow to Master Report DP 18	
FILING 1 MASTER DRAINAGE REPORT - STORM LINE D																													
D1	8	0.53	0.73	7.41	0.38	5.49	2.1									2.1	0.7	18	11.4	171	1.3	2.27						direct runoff to inlet at DP 8	
D3 Σ DP 8 + DP 9	9 10	0.32	0.64	6.96	0.20	5.64	1.1	9.68	0.59	4.83	2.8					1.1	5.0	18	30.5	36	3.4	0.18						direct runoff to inlet at DP 9 combined flow in storm sewer at DP 11	
D2 Σ DP 11 + DP 10	11 12	0.82	0.68	11.38	0.56	4.42	2.5	11.42	1.14	4.41	5.0					2.5	10.0	18	43.2	10	4.7	0.04						direct runoff to inlet at DP 11 combined flow in storm sewer at DP 11:	
D4 Σ DP 4 + DP 12		0.46	0.76	5.49	0.35	6.18	2.2	12.17	1.49	4.25	6.4					6.4	1.6	18	17.3	185	1.9	1.63						direct runoff to inlet at DP 13 combined flow in storm sewer at DP 11:	
D5 Σ DP 13 + DP 14	14 15	0.27	0.60	8.35	0.16	5.19	0.8	13.80	1.65	3.94	6.5					0.8	4.6	18	29.3	58	3.2	0.30						direct runoff to inlet at DP 14 combined flow in storm sewer at DP 11:	
D6 Σ DP 15 + D6		0.37	0.81	5.00	0.30	6.39	1.9	13.91	1.95	3.92	7.7					7.7	0.6	18	10.6	149	1.2	2.14						direct runoff to inlet at DP 16 combined flow in storm sewer at DP 11	
D7 Σ DP 16 + DP 17	17 18	0.46	0.71	6.43	0.32	5.82	1.9	16.05	2.28	3.57	8.1					1.9	4.5	18	29.0	81	3.2	0.42						direct runoff to inlet at DP 17 combined flow in storm sewer at DP 11:	
D8 Σ DP 8 + DP 18		0.58	0.64	11.48	0.37	4.40	1.6	16.92	2.65	3.45	9.1					9.1	0.5	18	9.7	44	1.1	0.69						direct runoff to inlet at DP 19 combined flow in storm sewer at DP 11:	
D9P2		0.77	0.60	14.16	0.46	3.88	1.8																					direct runoff to inlet at DP 19	
D8bP2	33	1.06	0.74	11.58	0.79	4.38	3.4	17.61	3.12	3.35	10.4																	total flow to Master Report DP 33	



Appendix D – Drainage Plan



NO CHANGES ARE TO BE MADE TO THIS DRAWING WITHOUT WRITTEN PERMISSION OF HARRIS KOCHER SMITH.

FILES: H:\P\2504026\ENGINEERING\DRAINAGE\DRAINAGE PLAN DWG LAYOUT LAYOUT1.DWG
FILES: H:\P\2504026\ENGINEERING\DRAINAGE\DRAINAGE PLAN DWG LAYOUT LAYOUT2.DWG
PLOTTED: THU 07/31/2025 12:33:38P BY: OLIVIA RILEY

LEGEND:

HAYMEADOW FILING 1 MASTER DRAINAGE REPORT, SEPTEMBER 2018 BY ALPINE ENGINEERING, INC

BASIN BOUNDARY
BASIN LABEL
DESIGN POINT

HAYMEADOW FILING 2 - PHASE 2 DRAINAGE CONFORMANCE LETTER, MAY 2025 BY HARRIS KOCHER SMITH

BASIN BOUNDARY
BASIN LABEL
DESIGN POINT

HAYMEADOW RMF 4 & 5 - PROPOSED

BASIN BOUNDARY
BASIN LABEL
DESIGN POINT



Know what's below.
Call before you dig.

40	0	40	80
SCALE: 1" = 40'			
DESIGNED BY: ORR			
CHECKED BY: RCP			
DRAWN BY: ORR			

ISSUE DATE: 07-31-2025	REVISION COMMENTS
DATE	

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HAYMEADOW HOMES, LLC

WILDFLOWER COTTAGES AT HAYMEADOW
PROPOSED DRAINAGE PLAN

PRELIMINARY
NOT FOR
CONSTRUCTION

PROJECT #: 2504026
SHEET NUMBER
1
1 OF 1